

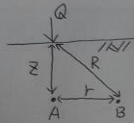
압밀 소요 시간

$$t = \frac{T_v H^2}{C_v}$$

$$50\% \rightarrow T_v = 0.197$$

$$90\% \rightarrow T_v = 0.848$$

$$C_v = \frac{k}{m_v \gamma_w} \quad m_v = \frac{a_v}{1+e_i}$$



$$\Delta \phi_A = \frac{3Q}{2\pi z^2}$$

$$\Delta \phi_B = \frac{3Q}{2\pi} \cdot \frac{z^3}{R^5}$$

$$\text{압축지수 } C_c = \frac{e_1 - e_2}{\log P_2 - \log P_1}$$

$$\text{불교란 } C_c = 0.009(W_L - 10)$$

$$\text{교란 } C_c = 0.007(W_L - 10)$$

점성토의 1차 압밀 침하량 S_c 점도증 9계P

$$\text{① 초기간극비법 } (e_0) \quad S_c = \frac{e_0 - e_1}{1 + e_0} H$$

$$\text{② 압축지수법 } (C_c) \quad S_c = \frac{C_c H}{1 + e} \log \frac{P + \Delta P}{P}$$

$$\text{③ 체적압축계수법 } (m_v) \quad S_c = m_v \cdot \Delta P \cdot H$$

$$t_1 : t_2 = H_1^2 : H_2^2$$

알은 기초

극한지지력 q_u

$$q_u = \alpha c N_c + \beta \gamma_i B N_r + \gamma_2 D_f N_q$$

$$\text{연속} - \alpha = 1.0 \quad \beta = 0.5$$

$$\text{점방} - \alpha = 1.3 \quad \beta = 0.4$$

허용지지력 q_a

$$q_a = \frac{Q_a}{A} = \frac{q_u}{F_s}$$

$$\text{극복전단 파괴의 점착력} = \frac{2}{3} c$$

B > d 일때

$$\gamma_1 = \gamma_{sub} + \frac{d}{B} (\gamma_t - \gamma_{sub})$$

깊은 기초

Meyerhof 극한지지력 $Q_u = Q_p + Q_f$
[t] 선단 마찰

$$Q_u = 40 N A_p + \frac{1}{5} \bar{N} A_s$$

$$N = N_3 \quad \bar{N} = \frac{N_1 h_1 + N_2 h_2 + N_3 h_3}{h_1 + h_2 + h_3}$$

$$A_p = \frac{\pi D^2}{4} \quad A_s = \pi D L$$

$$\text{허용지지력 } Q_a = \frac{Q_u}{F_s}$$

$$\text{부마찰력 } R_{nf} = f_s A_s = \frac{q_u}{2} \cdot \pi D L$$

히빙 현상

$$F_s = \frac{M_r}{M_d} = \frac{C_1 H R + C_2 \pi R^2}{\frac{R^2}{2} (\gamma_1 H + q)}$$

최대 굴착 깊이

$$\sigma = \gamma_{sat} (H-h)$$

$$u = \gamma_w \Delta h$$

$$\bar{\sigma} = \gamma_{sat} (H-h) - \gamma_w \Delta h$$

$\bar{\sigma} = 0$ 일때 히빙현상 발생하므로 h 가 최대굴착 깊이

$$\bar{\sigma} = 0 \Rightarrow h = \frac{\gamma_{sat} H - \gamma_w \Delta h}{\gamma_{sat}}$$

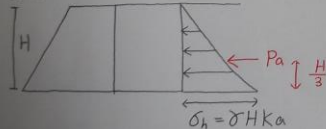
파이핑 현상

$$F_s = \frac{i_{cr}}{i} = \frac{\frac{\gamma_{sub}}{\gamma_w}}{\frac{H}{H+2d}}, \quad i_{cr} = \frac{C_v - 1}{1 + e}$$

주동도압) 모래 ($C=0$)

$$K_a = \frac{1 - \sin \phi}{1 + \sin \phi} = \tan^2 \left(45 - \frac{\phi}{2} \right)$$

① 일반식



$$P_a = \frac{1}{2} \gamma H^2 K_a \quad (\text{중이})$$

② 등분포 q

$$\sigma_h = (\gamma H + q) K_a$$

$$P_a = \gamma H K_a + \frac{1}{2} \gamma H^2 K_a$$

③ 지하수위

$$\sigma_h = \gamma_{sub} H K_a \quad u = \gamma_w H$$

$$P_a = \frac{1}{2} \gamma_{sub} H^2 K_a + \frac{1}{2} \gamma_w H^2$$

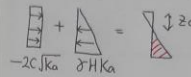
한계고

$$P_a = 0 \text{ 일때 } z_c = \frac{2C}{\gamma} \sqrt{K_p}$$

$$\text{인강균열 고려} \quad H_c = 2z_c = \frac{4C}{\gamma} \tan \left(45 + \frac{\phi}{2} \right)$$

$$\text{고려X} \quad H_c = \frac{2}{3} H_c$$

$$\sigma_h = \gamma H K_a - 2C \sqrt{K_a}$$



① 균열 발생 전

$$P_a = \frac{1}{2} \gamma H^2 K_a - 2CH \sqrt{K_a}$$

② 균열 발생 후 //

$$P_a = \frac{1}{2} (\gamma H K_a - 2C \sqrt{K_a}) (H - z_c)$$

③ 균열 발생 & 상재하중 고려

$$P_a = \frac{1}{2} \gamma (H - z_c)^2 K_a + \gamma z_c (H - z_c) K_a$$

옹벽의 안정

$$\text{전단) } F_s = \frac{W \cdot b + P_v \cdot B}{P_a \cdot L} \geq 2.0$$

$$\text{수평활동) } F_s = \frac{W \tan \phi + P_f}{P_a} \geq 1.5$$

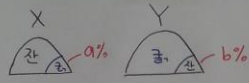
반 중력식 교대

$$\frac{\text{교대폭} - (\text{피복두께} \times 2)}{\text{배근 간격}} + 1$$

$$TF = LFT - (EST + D)$$

$$FF = EST_i - (EST_i + D)$$

골재량



$$X + Y = S + G \quad -①$$

$$\left(1 - \frac{a}{100}\right)X + \frac{b}{100}Y = S \quad -②$$

$$\text{잔골재량} \Rightarrow X = \frac{100S - b(S+G)}{100 - (a+b)}$$

$$\text{굵은골재량} \Rightarrow Y = \frac{100G - a(S+G)}{100 - (a+b)}$$

$$\text{단위수량} = W - W_s - W_a$$



콘크리트

$$V = \frac{W}{1000G}$$

$$G_c = \frac{\rho_c}{\rho_w}$$

단위골재 (S+G)의 절대체적

$$V_a = 1 - \left(\frac{\text{단위수량}}{1000} + \frac{\text{단위시멘트}}{C \text{ 비중} \times 1000} + \frac{\text{공기량}}{100} \right)$$

$$\text{단위 잔골재량} = V_a \times \text{잔골재율} \times \text{잔골재} \rho \times 1000$$

$$\text{잔골재량} = X + \text{잔골재의 표면수량}$$

배합강도 f_{cr}

$$\begin{aligned} &① \quad f_{ck} \leq 35 \quad f_{ck} > 35 \\ &\quad \left[\begin{aligned} &f_{ck} + 1.34S \\ &(f_{ck} - 3.5) + 2.33S \end{aligned} \right] \quad \left[\begin{aligned} &f_{ck} + 1.34S \\ &0.9f_{ck} + 2.33S \end{aligned} \right] \end{aligned}$$

큰 값

② 표준편차 모르거나 시험횟수 ≤ 14

$$\begin{aligned} &f_{ck} \quad f_{cr} \\ &X < 21 \quad f_{ck} + 7 \\ &21 \leq X \leq 35 \quad f_{ck} + 8.5 \\ &35 < X \quad 1.1f_{ck} + 5.0 \end{aligned}$$

③ 시험횟수 ≥ 29

직선보간으로 구한 보정계수 $\times S$ 해서 ① 식으로

$$\text{변동계수 } C_v = \frac{S}{\bar{X}} \times 100 (\%)$$

$\bar{X}$ 관리도

$$UCL = \bar{X} + A_2 \cdot \bar{R} \quad LCL = \bar{X} - A_2 \cdot \bar{R}$$

R 관리도

$$UCL = D_4 \cdot \bar{R} \quad LCL = D_3 \cdot \bar{R}$$

여유치

$$\text{양측: } \frac{|SU - SL|}{\delta} \geq 6 \quad \text{편측: } \frac{|SU - \bar{X}|}{\delta} \geq 3$$

공정능력 지수 C_p

$$\text{양측: } C_p = \frac{SU - SL}{6\delta} \quad \text{편측: } C_p = \frac{SU - \bar{X}}{3\delta}$$

건설기계

① 불도저

$$Q_D = \frac{60 \times Q \times f \times E}{C_m}, \quad f = \frac{1}{L}$$

② 리핑

$$Q_R = \frac{60 \times A_n \times l \times f \times E}{C_m}, \quad f = 1$$

③ 덤프트럭

$$Q = \frac{60 \cdot Q_t \cdot f \cdot E}{C_{mt}}, \quad Q_t = \frac{I}{\gamma_t} L$$

$$C_{mt} = \frac{n C_{ms}}{60 E_s} + t_2 + t_3 + t_4, \quad n = \frac{Q_t}{Q \times K}$$

④ 래머

$$Q = \frac{A N H f E}{\textcircled{P}} \rightarrow \text{되풀이 짚기 다감횟수}$$